

Structural Health Monitoring (SHM)
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Lecture – 29
Damage identification using lumped mass and
Element modal stiffness – Part 1

(Refer Slide Time: 00:20)

Module 2
 Lect 7: Damage identification using lumped mass.

Change in Eigenvalues - can enable damage detection

Let us consider dynamic eqn of forces acting @ the jth storey.
 By considering all the storeys above jth storey, we get:

$$K_{i,j}(t) d_{i,j}(t) + C_{i,j} \dot{d}_{i,j}(t) = f_{i,j}(t) - (1)$$

Friends, welcome to the lecture in Module 2, lecture 7. In this lecture, we will discuss about Damage identification using lumped mass modal. We already saw in the last lecture, change in Eigen values can enable damage deduction.

Let us now consider the dynamic equilibrium of forces acting at the jth storey, by considering all the storey above jth storey, we get $k_{i,j} d_{i,j} + c_{i,j} \dot{d}_{i,j} = f_{i,j}$ and say equation 1.

(Refer Slide Time: 02:18)

Where k_{ij} and c_{ij} are stiffness and damping parameters of the j^{th} storey

$f_{ij}(t)$ is the inertia force acting on the storey mass of j^{th} storey and all the storey above it.

$$f_{ij}(t) = - \sum_{k=j}^N m_k \ddot{x}_{ik}(t) \quad (2)$$

$$= - \omega_i^2 e^{\omega_i t} \sum_{k=j}^N m_k \phi_{ik} \quad (3)$$

where N - no. of storey
 m_k - mass of the k^{th} storey
 $d_{ij}(t)$ - is relative displacement b/w j^{th} & $(j-1)^{\text{th}}$ storey

Where, k_{ij} and c_{ij} are stiffness and damping parameters of the j^{th} storey; $f_{ij}(t)$ is the inertia force acting on the story mass of j^{th} storey and all the storey above it; $f_{ij}(t)$ is given by k equals j to N , $M_k \times \text{double dot } i \text{ of } t$ which is equal to minus $\omega_i^2 e^{\omega_i t}$ summation k equals j to N of $M_k \phi_{ik}$.

Where N is the number of storey, M_k is mass of the k^{th} storey $d_{ij}(t)$ is the relative displacement; that is very important; the relative displacement between j^{th} and $(j-1)^{\text{th}}$ storey.

(Refer Slide Time: 04:49)

for i^{th} mode of vibration, this can be expressed as

$$d_{ij}(t) = x_{ij}(t) - x_{i(j-1)}(t)$$

$$= \left\{ \phi_{i,j} - \phi_{i,j-1} \right\} e^{\omega_i t} \quad (4)$$

$$\dot{d}_{ij}(t) = \dot{x}_{ij}(t) - \dot{x}_{i(j-1)}(t)$$

$$= \left\{ \dot{\phi}_{i,j} - \dot{\phi}_{i,j-1} \right\} \omega_i e^{\omega_i t} \quad (5)$$

For i th mode of it is vibration, this can be expressed as d_{ij} of t is x_{ij} of t minus x_{ij} minus 1 of t which will be ϕ_{ij} minus ϕ_{ij} minus 1, $e^{\lambda_{ij} t}$; \dot{d}_{ij} of t is given by \dot{x}_{ij} of t minus \dot{x}_{ij} minus 1 t , which can be given by ϕ_{ij} minus ϕ_{ij} minus 1 of $\omega_{ij} e^{\omega_{ij} t}$. In fact, this should be also $\omega_{ij} t$.

Let us call this equation as 4 and 5.

(Refer Slide Time: 06:15)

When ϕ_{ij} & ω_{ij} - are eigenvector & eigenvalue of i 's mode

Once eigenvalue & eigenvector are measured for i 's storey,

Eq (1) can be formulated for j 's storey @ each time interval.

Using short data length, which is approximately equal to ω_{ij} in building.

Eq (1) can be solved.

one can repeat this procedure for other storey to determine $[k]$ & $[m]$ parameter of each storey.

Where, ϕ_{ij} and ω_{ij} are Eigenvector and Eigenvalue of i th mode. Once, the Eigenvector and Eigenvalue are measured for i th storey, equation 1 can be formulated for j th storey at each time interval using short data length which is approximately equal to natural frequency of the building; equation 1 can be solved.

One can repeat this procedure for other stories to determine stiffness and mass parameter of each storey.

(Refer Slide Time: 08:29)

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Salient points

- (1) This method assumes a known story mass
- (2) only applicable for lumped mass, shear building model

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There are some salient points of this method. The first would be, this method assumes a known story mass. This method is applicable for lumped mass, shear building model. Let us see the third method, method to identify damage using element modal stiffness.

(Refer Slide Time: 09:17)

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(3) Method to identify damage using Element Modal stiffness

In a linear, undamped structure, i 's modal stiffness is given by:

$$k_i = \bar{\phi}_i^T [k] \bar{\phi}_i \quad \text{--- (1)}$$

where $\bar{\phi}_i$ is the i 's mode shape vector

$[k]$ is the complete stiffness matrix of the entire structure

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In the linear undamped structure, i th modal stiffness is given by $k_i = \bar{\phi}_i^T \phi_i$ where, $\bar{\phi}_i$ is the i th mode shape vector, k is the complete stiffness matrix of the entire structure.

(Refer Slide Time: 11:24)

Combination of j^{th} member to i^{th} modal stiffness is given by:

$$k_{ij} = \bar{\phi}_i^T [k_j] \bar{\phi}_i$$

where k_j - j^{th} member contribution to $[k]$

Fraction of modal energy of i^{th} mode, contributed by j^{th} member is called Modal sensitivity.

Modal sensitivity is given by:

$$F_{ij} = \frac{k_{ij}}{k_i}$$

Now, combination of j th member to i th modal stiffness is given by k_{ij} is ϕ_i transpose k_j of ϕ_i where, k_j is the j th member stiffness, j th member contribution to the total K .

Now, fraction of modal energy of i th mode contributed by j th member is called Modal sensitivity. Modal sensitivity is given by F_{ij} is k_{ij} by k_i .

(Refer Slide Time: 13:25)

The above eqn is valid for undamaged structure.

This is modified for a damaged structure. Modal sensitivity for a damaged structure is given by:

$$F_{ij}^* = \frac{k_{ij}^*}{k_i^*}$$

$$k_{ij}^* = \bar{\phi}_i^{*T} [k_j^*] \bar{\phi}_i^*$$

$$k_i^* = \bar{\phi}_i^{*T} [k_i^*] \bar{\phi}_i^*$$

$$k_j = E_j [k_j]$$

$$k_j^* = E_j^* [k_j^*]$$

The above equation is valid for undamaged structure. This is modified for a damaged structure. The modal sensitivity for a damaged structure is given by, F_{ij}^* is k_{ij}^* by k_i^* .

by k_{ij} ; k_{ij} is $\bar{\phi}_i^T k_{ij} \bar{\phi}_i$ and k_{ij} is $\bar{\phi}_i^T k_{ij} \bar{\phi}_i$; k_{ij} is therefore, expressed as E_j of k_{ij} and k_{ij} is E_j of k_{ij} .

(Refer Slide Time: 15:22)

When E_j , E_j^* represent material stiffness properties, related to undamaged & damaged state of the structure

$[k_{j0}]$ is assumed to be unchanged even after damage occurs.

Basic assumption of this method is that

Modal sensitivity for i 's mode & j 's member remains unchanged before & after damage.

Mathematically

$$\frac{F_{ij}}{F_{ij}^*} = \frac{k_{ij}^*}{k_{ij}} \cdot \frac{k_i}{k_i} = 1$$

Where E_j and E_j star represent, material stiffness property related to undamaged and damaged state of the structure; k_{j0} is assumed to be unchanged even after damage occurs. This method has a basic assumption. The basic assumption of this method is that, Modal sensitivity for i th mode and j th member remain unchanged before and after damage.

Mathematically, F_{ij} by F_{ij}^* , that is damaged and undamaged state is k_{ij} star k_i by k_i star k_{ij} which is unity.

(Refer Slide Time: 17:39)

Damage Index, β_j for the j^{th} member is defined as:

$$\beta_j = \frac{E_j}{E_j^*}$$

Substituting from the earlier eqn

$$\beta_j = \frac{\gamma_{ij}^* k_i}{\gamma_{ij} E_i} = \frac{\bar{\phi}_i^{*T} [k_j] \bar{\phi}_i^* k_i}{\bar{\phi}_i^T [k_j] \bar{\phi}_i k_i^*}$$

Now, the damage index, beta j for the j th member is defined as beta j is E j by E star j. Substituting, from the earlier equations, beta j can be expressed as, i j star k i by i j k i star, which is phi bar i star transpose k j 0 phi bar star k i divided by phi bar i transpose k j phi bar i k i star.

(Refer Slide Time: 19:05)

Damage Index, can also be approximated as below.

$$\beta_j \approx \left[\frac{\bar{\phi}_i^{*T} [k_j] \bar{\phi}_i^* + \sum_{k=1}^{N_c} \bar{\phi}_i^{*T} [k_j] \bar{\phi}_i^*}{\bar{\phi}_i^T [k_j] \bar{\phi}_i + \sum_{k=1}^{N_c} \bar{\phi}_i^{*T} [k_j] \bar{\phi}_i} \right] \left(\frac{k_i}{k_i^*} \right)$$

Normalized damage indicator is given by:

$$z_j = \frac{\beta_j - \bar{\beta}}{\sigma_{\beta}}$$

where $\bar{\beta}$ mean value of β
 σ_{β} std deviation of β .

Now, further, the damage index can also be approximated as below; beta j is approximately equal to phi bar i star transpose k j naught phi bar i star plus summation of

k equals 1 to N E ϕ bar transpose k j 0 ϕ bar star divided by ϕ i transpose k j 0 ϕ i plus k equals 1 to N E ϕ i transpose k j 0 ϕ i multiplied by k i by k star i .

One can now obtain the normalized damage indicator is given by z j which is β j minus β bar by σ β where β bar is the mean value of the damage index.

(Refer Slide Time: 21:46)

Severity of damage can be estimated as:

$$E_j^* = E_j \left(1 + \frac{dE_j}{E_j} \right)$$

$$= E_j (1 + \alpha_j)$$

where $\alpha_j = \frac{\gamma_{ij} k_i^*}{\gamma_{ij}^* k_i}$

And σ β is the standard deviation of β . Severity of damage can be estimated as E j star is E j 1 plus d E j by E j which will be E j 1 plus α j where α j is γ i j k i star by γ i j star k i minus 1 .